DEPARTMENT OF TRANSPORTATION ENGINEERING SERVICE CENTER Transportation Laboratory 5900 Folsom Blvd. Sacramento, California 95819



## **UNCONFINED COMPRESSIVE STRENGTH OF SOIL**

### CAUTION:

Prior to handling test materials, performing equipment setups, and/or conducting this method, testers are required to read "SAFETY AND HEALTH" in Section H of this method. It is the responsibility of the user of this method to consult and use departmental safety and health practices and determine the applicability of regulatory limitations before any testing is performed.

### A. SCOPE

The unconfined compression test is used to measure the shearing resistance of cohesive soils which may be undisturbed or remolded specimens. An axial load is applied using either strain-control or stress-control condition. The unconfined compressive strength is defined as the maximum unit stress obtained within the first 20% strain.

### B. APPARATUS

The compression device is a hydraulic-actuated loading piston, electronically controlled, with the capability of infinite rates of strain and stress loads. A load cell of 44.5 kN or 222.5 kN capacity is fastened to the piston to measure load on specimen. Test data are displayed on control panel board readouts. See Figure 1 for compression device and Figure 2 for specimen holder.

### C. TEST RECORD FORM

Record test data on worksheet Form TL-265 (Figure 4). After test and calculations are completed, summarize test results on Form No. TL-239.

### D. PREPARATION OF SAMPLE

 No special trimming is required when specimens obtained with the California sampler are used in this test. Extrude the soil from the 101.6 mm high sample retainer in which it is received by pushing it in same direction it entered the tube, and then place sample directly into position on the loading device.

- 2. Trim soil which is received in a Shelby tube or a block sample to a 38.1 mm x 38.1 mm x 76.2 mm rectangular block specimen for testing, and do the trimming with the sample trimmer similar to the one shown in Figure 3.
- 3. Begin test immediately after the specimen is removed from the tube as drying will alter its characteristics considerably.



FIGURE 1



FIGURE 2

## E. TESTING PROCEDURE USING CONTROLLED STRAIN

Place specimen on base of holder, Figure 2, and set holder on compression device. Lower loading piston until it contacts specimen push rod. Contact will be noted by slight reading on load readout gauge. Zero out readout gauge, check strain rate setting, and begin test. Continue test until load values decrease or until 20% strain is reached. Make a moisture determination and also a sketch of specimen showing failure conditions such as slope angle, if measurable. Obtain classification tests as needed.

## F. CALCULATIONS

The following data are necessary for a complete test:

- 1. The boring samples and tube numbers.
- 2. A complete description of the specimen.
- 3. The moisture content.
- 4. The maximum load and the deflection due to that load.

The unconfined compressive strength is computed from the following formula:

qu = (P/A)(1-[?H/H])

Where:

qu = The unconfined compressive strength in kPa

P = Maximum load in kN.

A = Initial cross-sectional area of test specimen in square millimeters.

H = Initial height of test specimen.

? H = Reduction in height of test specimen.

1 - (? H/H) = Correction for increased area assuming constant volume.

#### G. NOTES

The unconfined compression test is usually made on undisturbed samples. It is reasonably simple and rapid to perform. It gives a very good measure of the shearing strength of cohesive soils. In somewhat granular soil its application is limited, but it does provide a good supplementary test for more complex strength tests.

The unconfined compression test is limited in that test conditions can be varied very little. Hence, the test may provide a good measure of the in-situ strength, but may provide only limited strength data, as the stress conditions change due to loading or construction.

## H. SAFETY AND HEALTH

Prior to handling, testing or disposing of any waste materials, testers are required to read: Part A (Section 5.0), Part B (Sections: 5.0, 6.0 and 10.0) and Part C (Section 1.0) of Caltrans' Laboratory Safety Manual.

Users of this method do so at their own risk.

## End of Text (4 Pages) on California Test 221

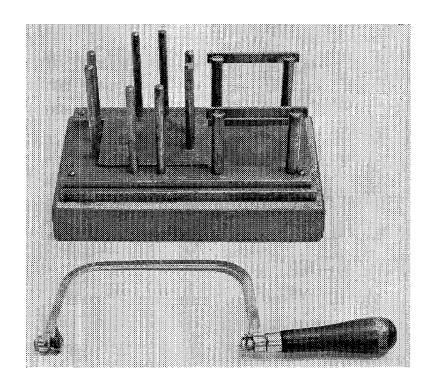


FIGURE 3

California Test 221 1978

# UNCONFINED COMPRESSION TEST TRANSPORTATION LABORATORY

20 800 20							NoSta. No	
%         Dial Reading         Calc. Actual Time         Axial Load           0         0         0         1/4 .010 .015 sec.            1/2         .020 .00 .00 .00 .00 .00 .00 .00 .00 .00	Job Name Job No Sample No.			Nominal Ht. 4 Initial Height Initial Diam.			Wet Wt. before Dry Wt. after Specific Gravity	
0 0 0 1/4 .010 15 sec. 1/2 .020 30 3/4 .030 45 1 .040 1 min. 1 -1/2 .060 1 -1/2 2 .080 2 .080 2 .02 .1/2 .100 2 -1/2 .3 1 .120 3 .3 .120 3 .3 .1/2 .140 3 -1/2 .4 .160 4	/- I		1				A. T. Moist %	
1/4	Strain	Reading	Time	Time	Load		1	
3/4	1/4	. 010 . 020	30					
1-1/2	3/4	.040	l min.					
3   120   3   3   1   3   1   1   4   1   1   1   1   1   1   1	2	. 080	2		<b> </b>			
A   . 160   4	3	. 120	3					
S			<u> </u>					
6								
8 .320 8 9 .360 9 10 .400 10 12 .480 12 14 .560 14 16 .640 16 18 .720 18 20 .800 20    M. A	6	. 240	6					
10	8	. 320						
12					<b> </b>		1 14 10 5	
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18       .720       18					1			
3F33  Moisture: DryMoist Wet Free Water  Homo Hetero Band Strat Lamin Lenses Cement Fiss  Structure:		. 720	18		1		Other Tests	
Moisture: Dry Moist. Wet Free Water  Homo Hetero Band Strat Lamin Lenses Cement Fiss  Calc Mott Roots Voids Others HCL Reaction  Consistency: Very soft Soft Firm Stiff Hard Rock  Visual Class Color	20	. 800	20					
Structure: Homo Hetero Band Strat Lamin Lenses Cement Fiss Calc Mott Roots Voids Others HCL Reaction  Consistency: Very soft Soft Firm Stiff Hard Rock Visual Class Color					3E33			
Structure: Calc Mott Roots Voids Others HCL Reaction  Consistency: Very softSoft FirmStiff Hard Rock  Visual Class Color	Moisture	: Dry	Mois	tWe	tFree W	ater		
Visual Class Color	Structure:							
		•						
							Color	

TL-265D(Rev.9/75)

FIGURE 4